



Budapest University of Technology and Economics

Department of Mechanics, Materials and Structures

English courses

Reinforced Concrete Structures

Code: BMEEPSTK601

Lecture no. 8:

ONE-WAY SLABS, STAIR SLABS

Content:

I. One-way slabs

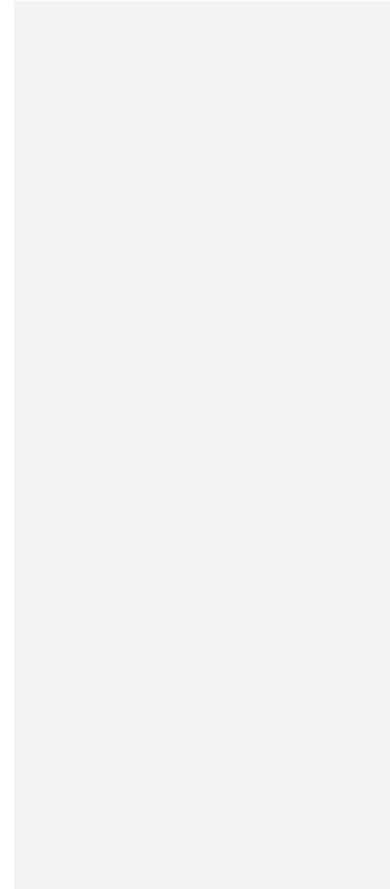
1. Definition of one-way and two-way slabs
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 4. Fulfilment of the rigidity requirement of slabs
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 6. Reinforcement system of simple supported and continuous one-way slabs, the distribution steel
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II. Stair slabs

1. Static models
2. Substitutive static model of a two-flight staircase in flight direction
3. System of reinforcement

Formázott: Automatikusan sorszámozás
+ Szint: 1 + Számozás stílusa: 1, 2, 3,
... + Kezdő sorszám: 1 + Igazítás: Bal
oldalt + Igazítás: 1,25 cm +
Tabulátorhely: 1,89 cm + Behúzás:
1,89 cm

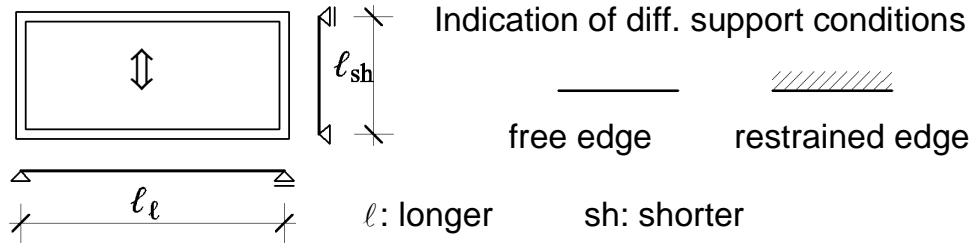
4. Stairs spanning transversally
5. Geometry of landing with and without transverse beam



I. One-way slabs

1. Definition of one-way and two-way slabs

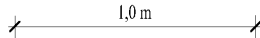
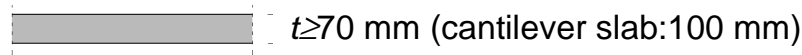
Rectangular slab panel simply supported along the perimeter:



If $\frac{l}{l_{sh}} \geq 2$, the slab is regarded one-way slab, otherwise two-way slab

2. Special characteristics of rc slabs

-for convenience, 1 m wide strip of the slab is investigated:



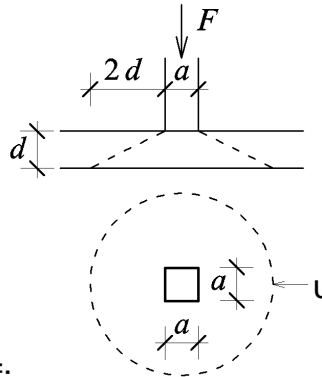
-with the exception of introduction of important concentrated loads at column heads of flat slabs or column support points on foundation slabs, *no shear reinforcement* is needed:

$$u = 4a + 2 \cdot 2d\pi$$

$$v_{Rd,c} = c f_{dt,d} \quad c \text{ tabulated in DA}$$

Shear reinforcement must be designed only, if:

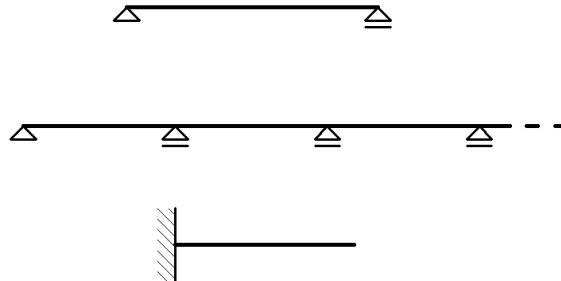
$$F \geq v_{Rd,c} u d$$



3. Static models

The way of determination of the *position of the support points* is the same as for beams with $h=t$ (slab thickness)

Static models:



4. Fulfilment of the rigidity requirement of slabs

Slabs are flexible, ductile structures. At large (not allowable) deflections the suspension effect may impede rupture and fall down of slabs.

$\left(\frac{\ell/K}{d}\right)_{\text{allowable}}$ rates range from 20 to 40 (see DA table) and can be

effected by:

-over reinforcing ($A_{s,\text{prov}} \succ A_{s,\text{req}}$)

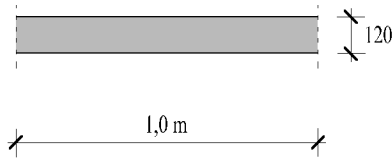
-pre-camber (overlifting) by $\frac{\ell}{250}$ or $\frac{\ell}{500}$

-applying restraint at the support (that is increasing K)

-prescribing higher concrete grade

-increasing slab thickness

5. Section design for moment (numerical example)



Concrete: C20/25-X0-24-F3

Steel: C15.H welded mesh

Concret cover: $c_{nom} = 20$ mm

$m_{Ed} = -12$ kNm/m (- means tension on top!)

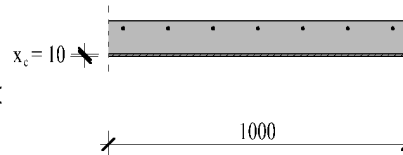
Design the necessary steel section!

Solution:

$$d = 120 - 20 - 10/2 = 95 \text{ mm (no link diameter subtracted!)}$$

$$\underline{\Sigma M_s = 0}: x_c = d \left(1 - \sqrt{1 - \frac{2m_{Ed}}{bd^2 f_{cd}}} \right), \quad b = 1000 \text{ mm}, \quad f_{cd} = 13,3 \text{ N/mm}^2$$

$$x_c = 95 \cdot \left(1 - \sqrt{1 - \frac{2 \cdot 12 \cdot 10^6}{1000 \cdot 95^2 \cdot 13,3}} \right) = 10,0 \text{ mm} <$$



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$$< x_{co} = \xi_{co} d = 0,49 \cdot 95 = 46,6 \text{ mm OK!}$$

$$z = d - \frac{x_c}{2} = 95 - 10/2 = 90 \text{ mm}$$

$$\underline{\Sigma M_c = 0}: a_s f_{yd} \cdot z - m_{Ed} = 0, f_{yd} = 435 \text{ N/mm}^2$$

$$a_s = \frac{m_{Ed}}{f_{yd} \cdot z} = \frac{12 \cdot 10^6}{435 \cdot 90} = 306,5 \text{ mm}^2/\text{m} > a_{s,\min} \text{ OK!}$$

$$a_{s,\min} = \rho_{\min} \cdot bd = \frac{1,3}{1000} \cdot 1000 \cdot 95 = 123,5 \text{ mm}^2/\text{m}$$

Let use $\text{Ø}8,2/150$ ($a_s = 352,1 \text{ mm}^2$) intensity welded mesh!

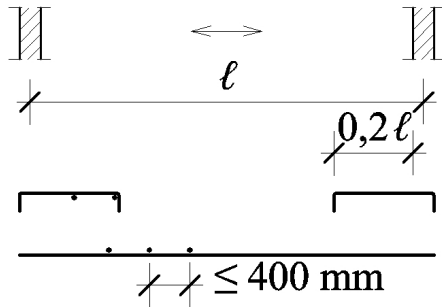
Ccheck of further constructional rules:

$$t \leq 150 \text{ mm} \text{ esetén } s_{\max} = 150 \text{ mm, rendben!}$$

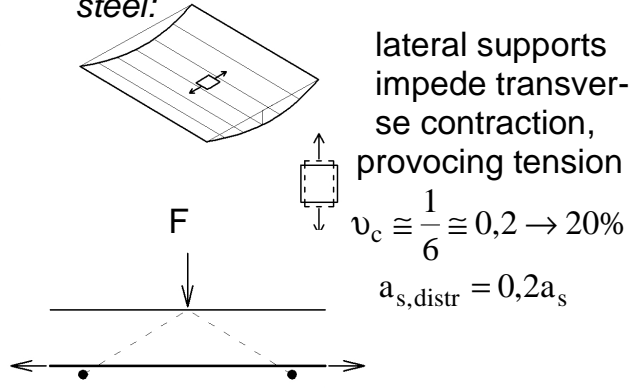
$$\text{Ø}_{\max} \leq \frac{t}{10} = \frac{120}{10} = 12 \text{ mm, OK!}$$

6. Reinforcement system of simple supported and continuous one-way slabs, the distribution steel

Simple supported slab
Plan detail

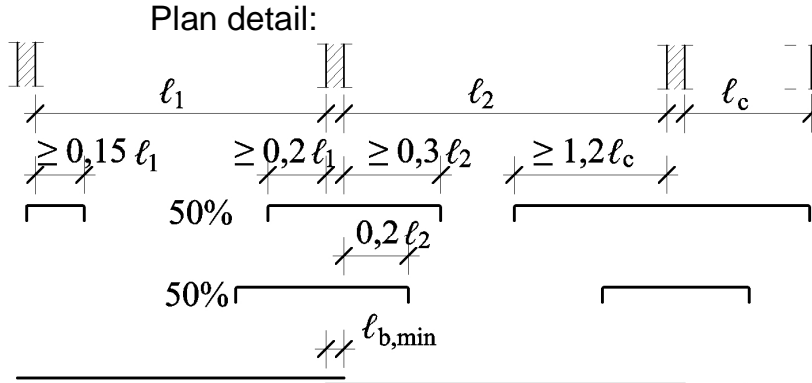


Reasons and quantity of *distribution steel*:



Distribution steel is also needed to distribute effect of uneven (concentrated) loads

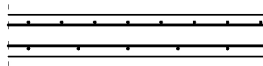
Continuous slab:



When respecting the rules indicated on the figure, no enveloping of the extreme applied moment diagram is needed.

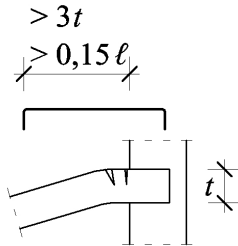
standees to support top steel

Correct order of the layers of main and distribution steel on cross-section at intermediate support:

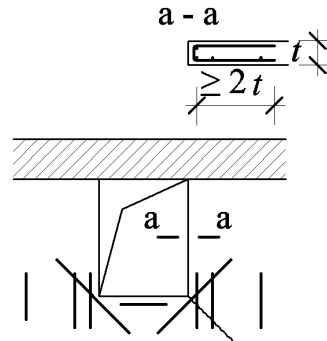


7. Special reinforcement details: anti-crack reinforcement, free-edge reinforcement, additional reinforcement at holes and under linear loading

anti-crack reinforcement along lateral supports of one-way slabs:

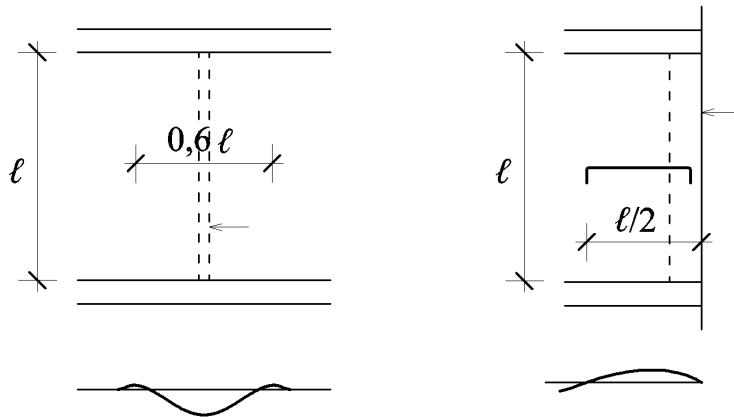


Free edge reinforcement and elements of additional reinforcement at holes:



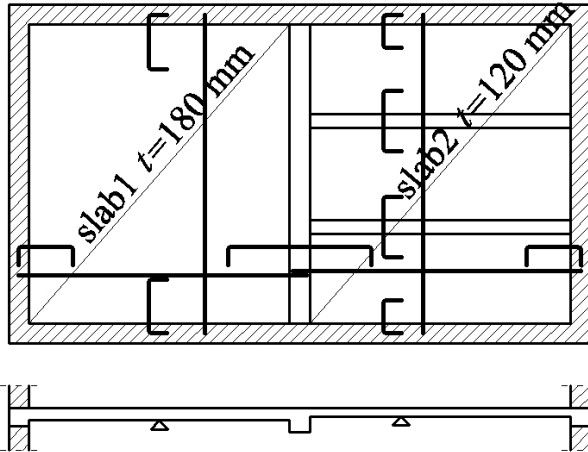
danger of diagonal cracking!

Local strengthening of the slab reinforcement needed for moments due to linear (or concentrated) loading of heavier partition wall, facade wall etc.



8. Example of a floor

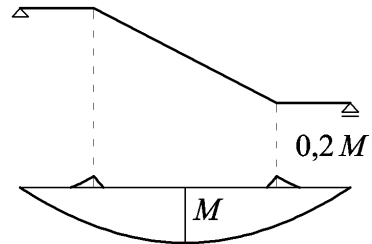
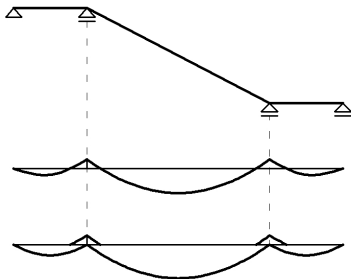
Elements of the reinforcement to be designed for the slab indicated on the structural plan below:



II. Stair slabs

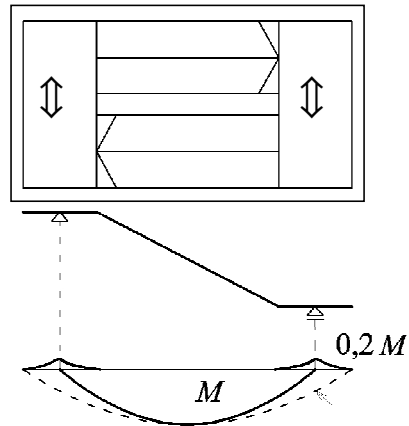
1. Static models

Acceptable static models and design moment diagrams



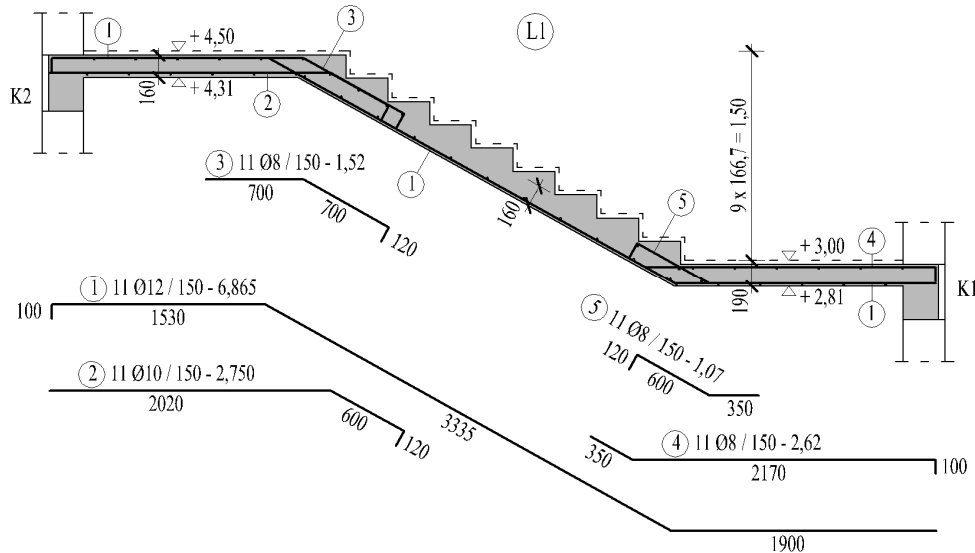
2. Substitutive static model of a two-flight staircase in flight direction

Landing slabs can be regarded as wide supports, considering the support line along the axis of them: this approximation reduces significantly moments in flight direction. (The practice has proved this approximation)



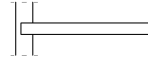
3. System of reinforcement

Elements of the reinforcement system from numerical example of a two flight staircase

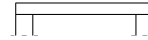


4. Stairs spanning transversally

Stair restrained in (rc) wall



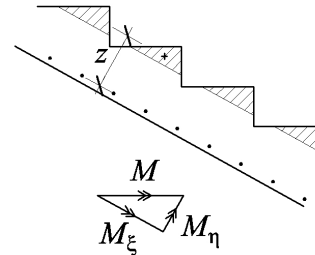
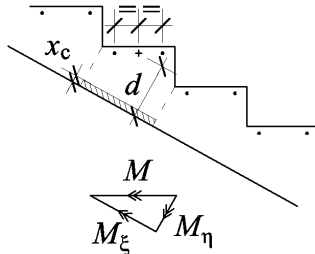
supported by parallel walls



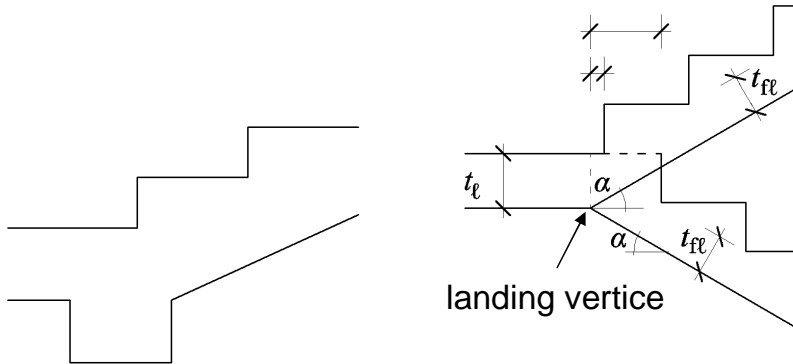
supported by parallel stringer beams



The way of flexural design for negative and positive moments:



5. Geometry of landing with and without transverse beam



Elaboration of details like this requires intensive cooperation of the architect and the structural designer.